HELICAL \mathbb{R} PULLDOWN® A CASE HISTO **Micropiles Report**

Project:

Geotechnical Consultant: Condominium Building Soil Mat Engineers and Richmond Hill, Ontario Consultants Hamilton, ON

Consultant: The SPG Engineering

Contractor: EBS Engineering Group Ltd. Oakville, ON Construction Breslau, ON

The Problem:

An 11-story condominium building north of Toronto had been constructed less than 10 years on a dense clay till capping an artesian aquifier. Large openbottom sumps (installed during the initial construction to control groundwater) had undermined four shear walls, two columns and the parking garage foundation walls on the southwest corner of the structure and had disturbed the dense sand layer under the clay till.

Considering the limited access in the parking garage and the artesian aquifier, pile driving and caisson installation were not viable underpinning options.

The Solution:

Chance Helical Pulldown® Micropiles were determined to comprise the best solution to underpin the structure. An ASTM D 1143-S1, 24-hour standard test method for piles under static axial compressive load was completed prior to project startup to confirm the capacity of the Helical Pulldown[®] Micropiles.

The Helical Pulldown[®] Micropile size and configuration used was the SS200 with 8", 10" and 12" diameter helices. A 6" diameter grout column around the shaft was sleeved with PVC pipe above the helices to increase lateral stability and prevent negative skin friction on the piles if the clay till were to continue to settle. Installation depths varied from 34 to 44 feet on the 83 piles installed to a 60-kips minimum allowable working load or a 120-kips minimum ultimate load capacity.

The Results:

Four times a year, the structure has been monitored for movement since the project was completed. No settlement movement has been recorded since the structure was underpinned with Chance Helical Pulldown[®] Micropiles.



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